

FacadeMatrix

GCC WIND LOAD & Ixx CALCULATOR

SAMPLE CALCULATION REPORT

Dubai Office Tower · ASCE 7-16 · Chapter 30

REPORT REFERENCE

SR-GCC-001

26 May 2026

Reference Case

Dubai · $h = 60 \text{ m}$ · Exposure C · $b = 1.5 \text{ m}$ · $L = 3.5 \text{ m}$

This sample report is generated for demonstration purposes only. Calculations follow ASCE 7-16 Chapter 30 (C&C). FacadeMatrix accepts no liability for design decisions made using this sample. Engineers must independently verify all values before use in any submission.

facadematrix.com

Input Parameters

City	Dubai
Country	UAE
Design code	ASCE 7-16 · Chapter 30 (C&C)
Applicable figure	Figure 30.4-1 (h > 18.3 m / 60 ft)
Enclosure classification	Enclosed building
Basic wind speed V	45 m/s (3-sec gust · Risk Cat II · 10 m AGL)
Mean roof height h	60.00 m / 196.85 ft
Exposure Category	C — open desert / inland terrain
Least plan dimension B_min	40.00 m
Mullion span L	3.50 m
Bay width b	1.50 m

Material Properties — AA 6063-T6 Aluminium

Material	AA 6063-T6 Extruded Aluminium Alloy
Modulus of elasticity E	69,000 MPa (68.9 GPa — Thomasnet)
Yield strength Fy	214 MPa
Allowable bending stress sigma_allow	128 MPa (ADM ASD · 0.6 x Fy)
Deflection code basis	AAMA TIR-A11 / IBC §2403.3 / CWCT TN 32

Code & Standard References

Wind load procedure	ASCE 7-16 Chapter 30 — C&C envelope procedure
Velocity pressure equation	ASCE 7-16 §26.10 — $q_h = 0.613 \cdot K_h \cdot K_{zt} \cdot K_d \cdot K_e \cdot V^2$
G _{Cp} figure (h > 60 ft)	ASCE 7-16 Figure 30.4-1 — Walls, high-rise buildings
Internal pressure G _{Cpi}	ASCE 7-16 Table 26.13-1 — ±0.18 (enclosed building)
Minimum design pressure	DM Res. 37/2021 §2.4 — 1.0 kN/m ² floor (Dubai)
Deflection — L/175	AAMA TIR-A11 — spans ≤ 4.1 m (13 ft 6 in)
Deflection — L/240 + 6.35 mm	AAMA TIR-A11 — spans 4.1 m to 12 m
Material design standard	ADM Aluminum Design Manual (ASD) — sigma_allow = 0.6 x Fy = 128 MPa

Calculation Steps 1–20

Site Parameters · Velocity Pressure · Zone a · G_{Cp} · Design Pressures · Mullion Section Properties

Step 1 — Basic Wind Speed	
FORMULA	V from Dubai Wind Code 2013 §3.1.2 via ASCE 7-16
VALUES	3-sec gust at 10 m AGL · Risk Category II · Exposure C
RESULT	V = 45 m/s

Dubai Wind Code 2013 §3.1.2: "30 m/s (10-min mean) ≅ 45 m/s (3-sec gust)"

Step 2 — Building Height Conversion

FORMULA	$h_{ft} = h_m \times 3.28084$
VALUES	$h_{ft} = 60.00 \times 3.28084 = 196.85 \text{ ft}$
RESULT	$h = 60.00 \text{ m} / 196.85 \text{ ft}$

Step 3 — Applicable ASCE 7-16 Figure

FORMULA	Criterion: $h > 18.3 \text{ m} (60 \text{ ft}) \rightarrow$ Figure 30.4-1 applies GCpi = ±0.18 (ASCE 7-16 Table 26.13-1 — enclosed building)
VALUES	$h = 60.00 \text{ m} > 18.3 \text{ m} \rightarrow$ high-rise wall C&C procedure applies
RESULT	Applicable figure: ASCE 7-16 Figure 30.4-1

Step 4 — Velocity Pressure Coefficient Kh

FORMULA	$K_h = 2.01 \times (h / z_g)^{2/\alpha}$ Exposure C: $z_g = 274.32 \text{ m} \cdot \alpha = 9.5$
VALUES	$z_{eff} = \max(60.00, 4.57) = 60.00 \text{ m}$ $K_h = 2.01 \times (60.00/274.32)^{2/9.5} = 2.01 \times 0.7265$
RESULT	$K_h = 1.460$

Computed from formula per ASCE 7-16 §26.10. Table interpolation not used.

Step 5 — Pressure Modification Factors

FORMULA	Kzt per §26.8 Kd per Table 26.6-1 Ke per §26.9
VALUES	Kzt = 1.00 — flat terrain, no topographic feature Kd = 0.85 — buildings, components & cladding (C&C) Ke = 1.00 — sea-level site
RESULT	$K_{zt} = 1.00 \mid K_d = 0.85 \mid K_e = 1.00$

Step 6 — Velocity Pressure qh

FORMULA	$q_h = 0.613 \times K_h \times K_{zt} \times K_d \times K_e \times V^2 \text{ (N/m}^2\text{)}$
VALUES	$q_h = 0.613 \times 1.460 \times 1.00 \times 0.85 \times 1.00 \times 45^2$ $= 0.613 \times 1.460 \times 0.85 \times 2,025$
RESULT	$q_h = 1,540 \text{ N/m}^2 = 1.540 \text{ kN/m}^2$

Step 7 — Zone 5 Corner Dimension a

FORMULA	$a = \min(0.1 \times B_{min}, 0.4 \times h) \geq \max(0.04 \times B_{min}, 0.91 \text{ m})$
VALUES	$a = \min(0.1 \times 40, 0.4 \times 60) = \min(4.00, 24.00) = 4.00 \text{ m}$ Min check: $\max(0.04 \times 40, 0.91) = \max(1.60, 0.91) = 1.60 \text{ m} \checkmark$
RESULT	Zone 5 a = 4.00 m from each external building corner

Zone 4 = field zone (beyond a). Zone 5 = corner strip of width a.

Step 8 — Effective Wind Area Aeff

FORMULA	$A_{trib} = b \times L \mid A_{1/3} = L \times (L/3) \mid A_{eff} = \max(A_{trib}, A_{1/3})$
---------	--

VALUES $A_{trib} = 1.50 \times 3.50 = 5.25 \text{ m}^2$ | $A_{1/3} = 3.50 \times 1.167 = 4.08 \text{ m}^2$
 $A_{eff} = \max(5.25, 4.08) = 5.25 \text{ m}^2 = 56.53 \text{ sq ft}$

RESULT **$A_{eff} = 5.25 \text{ m}^2 (56.53 \text{ sq ft})$**

ASCE 7-16 §C26.6: effective wind area need not be less than $LxL/3$ for slender members.

Step 9 — GCp — Zone 4 (Figure 30.4-1, log-linear interpolation)

FORMULA $GC_p(A) = GC_{p_20} + (GC_{p_500} - GC_{p_20}) \times [\ln(A) - \ln(20)] / [\ln(500) - \ln(20)]$
 Anchors: $A=20 \text{ sq ft} \rightarrow \pm 0.90$ | $A=500 \text{ sq ft} \rightarrow \pm 0.60$

VALUES $A=56.53 \text{ sq ft}$: $GC_p = 0.90 + (0.60 - 0.90) \times [\ln(56.53) - \ln(20)] / [\ln(500) - \ln(20)]$
 $= 0.90 - 0.30 \times [4.034 - 2.996] / [6.215 - 2.996] = 0.90 - 0.097$

RESULT **$+GC_p \text{ Zone 4} = +0.803$ | $-GC_p \text{ Zone 4} = -0.803$**

Step 10 — GCp — Zone 5 (Figure 30.4-1)

FORMULA $+GC_{p5} = +GC_{p4}$ (same positive curve)
 $-GC_p$: anchors $A=20 \text{ sq ft} \rightarrow -1.80$ | $A=500 \text{ sq ft} \rightarrow -1.00$

VALUES $-GC_{p5} = -1.80 + (-1.00 - (-1.80)) \times [\ln(56.53) - \ln(20)] / [\ln(500) - \ln(20)]$
 $= -1.80 + 0.80 \times 0.3225 = -1.80 + 0.258$

RESULT **$+GC_p \text{ Zone 5} = +0.803$ | $-GC_p \text{ Zone 5} = -1.542$**

Zone 5 suction (-1.542) is 92% higher than Zone 4 (-0.803) — corner suction governs.

Step 11 — Design Pressure Equation [ASCE 7-16 Eq. 30.4-1]

FORMULA $p = q_h \times (GC_p - GC_{pi})$

VALUES $q_h = 1.540 \text{ kN/m}^2$ | $GC_{pi} = \pm 0.18$ (enclosed building)
 Four combinations evaluated for Zone 4 and Zone 5.

RESULT **See Steps 12 and 13 for full combination tabulation.**

Step 12 — Zone 4 Pressure Combinations (field zone)

Combination	Formula	Calculation	p (kN/m ²)
1 — Inward	$q \times (+GC_p - (-GC_{pi}))$	$1.540 \times (0.803 + 0.18)$	+1.514
2 — Suction	$q \times (-GC_p - (+GC_{pi}))$	$1.540 \times (0.803 + 0.18)$	-1.514
3	$q \times (+GC_p - (+GC_{pi}))$	$1.540 \times (0.803 - 0.18)$	+0.960
4	$q \times (-GC_p - (-GC_{pi}))$	$1.540 \times (0.803 - 0.18)$	-0.959
Governing Zone 4			$\pm 1.514 \text{ kN/m}^2$

Step 13 — Zone 5 Pressure Combinations (corner zone — governs)

Combination	Formula	Calculation	p (kN/m ²)
1 — Inward	$q \times (+GC_p - (-GC_{pi}))$	$1.540 \times (0.803 + 0.18)$	+1.514
2 — Suction ★	$q \times (-GC_p - (+GC_{pi}))$	$1.540 \times (1.542 + 0.18)$	-2.652
3	$q \times (+GC_p - (+GC_{pi}))$	$1.540 \times (0.803 - 0.18)$	+0.960
4	$q \times (-GC_p - (-GC_{pi}))$	$1.540 \times (1.542 - 0.18)$	-2.097
Governing Zone 5		SUCTION GOVERNS	-2.652 kN/m²

Pressure Envelope Summary

Zone	+GCp	-GCp	p_pos (kN/m ²)	p_neg (kN/m ²)
Zone 4 (field)	0.803	0.803	+1.514	-1.514
Zone 5 (corner)	0.803	1.542	+1.514	-2.652

Governing design pressure: p_design = 2.652 kN/m² (Zone 5 suction · Combination 2)

Dubai Municipality minimum pressure floor: 1.0 kN/m² (DM Res. 37/2021 §2.4) — **not triggered**. Calculated 2.652 kN/m² governs.

Step 14 — Mullion Line Load w

FORMULA	$w \text{ [N/mm]} = p_{\text{design}} \text{ [kN/m}^2\text{]} \times b \text{ [m]}$ (1 kN/m = 1 N/mm exactly)
VALUES	$w = 2.652 \times 1.50$
RESULT	w = 3.978 kN/m = 3.978 N/mm

Step 15 — Maximum Bending Moment M

FORMULA	$M = w \times L^2 / 8$ (simply supported · uniformly distributed load)
VALUES	$M = 3.978 \text{ [N/mm]} \times 3,500^2 \text{ [mm}^2\text{]} / 8 = 3.978 \times 12,250,000 / 8$
RESULT	M = 6,092,625 N-mm = 6.093 kN-m

Step 16 — Maximum Shear Force V

FORMULA	$V = w \times L / 2$
VALUES	$V = 3.978 \times 3,500 / 2 = 6,962 \text{ N}$
RESULT	V = 6,962 N = 6.962 kN

Step 17 — Required Section Modulus Z_req

FORMULA	$Z_{\text{req}} = M / \sigma_{\text{allow}}$ [$\sigma_{\text{allow}} = 128 \text{ MPa} \cdot \text{ADM ASD} \cdot 0.6 \times F_y$]
VALUES	$Z_{\text{req}} = 6,092,625 \text{ N-mm} / 128 \text{ N/mm}^2$
RESULT	Z_req = 47,598 mm³ = 47.6 cm³

Select extrusion with section modulus $Z \geq 47,598 \text{ mm}^3$ about the strong axis.

Required Moment of Inertia Ixx — Three Deflection Limits

Common formula: $I_{xx_req} = 5 \times w \times L^4 / (384 \times E \times \delta_{\text{allow}})$ where $E = 69,000 \text{ N/mm}^2$ (AA 6063-T6 · Thomasnet) · all in N and mm.

Step 18 — Ixx at L/175 — Recommended (L ≤ 4.1 m · AAMA TIR-A11)

FORMULA	$\delta_{\text{allow}} = L/175 = 3,500/175 = 20.00 \text{ mm}$
VALUES	$I_{xx} = 5 \times 3.978 \times 3,500^4 / (384 \times 69,000 \times 20.00)$ $= 5 \times 3.978 \times 1.5006 \times 10^{13} / 530,496,000$
RESULT	Ixx = 5,633,500 mm⁴ = 563.35 cm⁴ ← RECOMMENDED

Step 19 — Ixx at L/240 + 6.35 mm — Long-span limit (AAMA TIR-A11)

FORMULA	$\delta_{\text{allow}} = 3,500/240 + 6.35 = 14.583 + 6.35 = 20.933 \text{ mm}$
VALUES	$I_{xx} = 5 \times 3.978 \times 3,500^4 / (384 \times 69,000 \times 20.933)$
RESULT	Ixx = 5,382,300 mm⁴ = 538.23 cm⁴

L/240+6.35 governs for spans 4.1 m to 12 m. At L=3.5 m, L/175 governs.

Step 20 — Ixx at L/360 — Project-specific (SSG / brittle finishes)

FORMULA	$\text{delta_allow} = 3,500/360 = 9.722 \text{ mm}$
VALUES	$I_{xx} = 5 \times 3.978 \times 3,500^4 / (384 \times 69,000 \times 9.722)$
RESULT	$I_{xx} = 11,588,800 \text{ mm}^4 = 1,158.88 \text{ cm}^4$

Use L/360 only when project specification explicitly requires it (ETAG 002 / EN 13022).

Deflection Limit Summary — AA 6063-T6 Mullion

Deflection Limit	delta_allow (mm)	Ixx required (cm ⁴)	Note
L / 175	20.00	563.35	RECOMMENDED
L / 240 + 6.35 mm	20.93	538.23	
L / 360	9.72	1,158.88	PROJECT-SPECIFIC

Results Summary

Velocity pressure qh	1.540 kN/m ²
Zone 5 corner dimension a	4.00 m
Effective wind area Aeff	5.25 m ² (56.53 sq ft)
Governing design pressure p_design	2.652 kN/m ² — Zone 5 suction governs
Mullion line load w	3.978 kN/m = 3.978 N/mm
Maximum bending moment M	6.093 kN-m
Maximum shear force V	6.962 kN
Required section modulus Z_req	47,598 mm ³ (47.6 cm ³)
Ixx at L/175 (RECOMMENDED)	563.35 cm ⁴
Ixx at L/240 + 6.35 mm	538.23 cm ⁴
Ixx at L/360 (project-specific)	1,158.88 cm ⁴

Design Recommendation

Select an AA 6063-T6 extruded aluminium mullion with $I_{xx} \geq 563.35 \text{ cm}^4$ (L/175 · AAMA TIR-A11 · recommended for spans $\leq 4.1 \text{ m}$ per IBC §2403.3) and section modulus $Z \geq 47,598 \text{ mm}^3$. Apply 2.652 kN/m² for corner zones within 4.00 m of each external corner (Zone 5). Apply 1.514 kN/m² for field zones (Zone 4).

FacadeMatrix accepts no liability for design decisions made without independent verification by a licensed structural or facade engineer. Wind-tunnel testing mandatory for h > 120 m (QCS 2014 §1.5.2). FacadeMatrix · facadematrix.com · SR-GCC-001 · 26 May 2026